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December 2, 2020
Daniel W. Kops, Jr., Town Planner
Town of Hamden
2750 Dixwell Avenue
Hamden, Connecticut 06518

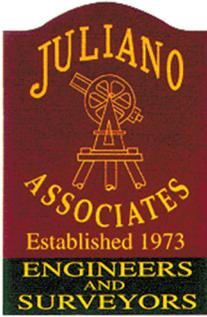
Subject: Special Permit Application 20-1511/WS - Restoration of Adjacent Properties
Response to Town Comments
82-92 Crest Way, 785 Sherman Avenue, 925 Sherman Avenue

Dear Mr. Kops,

This letter is being submitted on behalf of Eighty-Two Ninety-Two Crest Way, LLC, the owner of the 82-92 Crest Way property and the applicant for the subject project.

I would like to thank you as well as Mr. Mark Austin and Attorney Tim Lee for meeting with members of the design team as well as myself on 12/1/20. This meeting did provide us with some valuable insight on some of the concerns you and Mr. Austin have. Below are a few comments which we would like to formally address to hopefully provide a smooth meeting with the Planning and Zoning Commission. As the meeting is rapidly approaching, we felt a letter was appropriate.

1. It was requested that more detail on the 100 Overlook Drive encroachment along with the remediation of work be more documented. In an effort to better document this encroachment the linework and spot elevations as was presented on the 11/5/19 survey regarding this encroachment would be added to the updated survey. This encroachment has been brought back to its original state in January of 2020. This is why the contours shown match what was original from the 2015 property survey.
2. Now further understanding Mr. Austins comment regarding the material composition of the "excavator pad" we now understand that the concern is pad stability. In order to promote additional safety to alleviate concerns the excavator will now over excavate the pad by an additional 3'. Once it is determined that there are no large trees and voids the soil material will then be placed back to create the pad. As the depth from the top of pad to "original grade" is approximately 6' this should eliminate the concerns of trees and voids and provide a stable area for the operator and the machine.
3. Swale calculations have been completed and show that the swale depth and width are more than adequate for handling the contributing areas stormwater. The Hydrocad report is also attached to this document. The max velocity leaving this swale will be 0.52 ft/s in



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the 10-year storm. Due to the velocities at the end of the swale we are proposing a plunge pool. Detail information on the plunge pool is shown on sheet 3 of the submission set (11/30/20) and the size will match as if a 15" pipe was utilized.

4. The existing detention basin was not finalized as per the approved plan as the cease and desist order has prevented additional work from being completed. As it stands it is not a sufficient size for stormwater and sediment. We will propose that this detention basin will be finalized in terms of volume as per the approved plans (see sheet 3 on 11/30/20 plan set for detention basin details). This will be added to the proposed phasing plan in phase 1.
5. A concern about the ability for vehicles as well as machines to be able to traverse the proposed swale. This proposed swale is a shallow 6" swale that will be 6' wide. This is easily traversable by all equipment that will be utilized on site. There is also a note on the 11/30/20 plan set that notes that this swale will need to be inspected and repaired at the end of every day. As the excavation process will not be conducted during a rain event, for safety concerns, this swale will always be ready to handle water when the rain event happens.
6. The standard town of Hamden note will be added to the plan set. "Any damaged/broken sidewalks and driveway aprons will be replaced and repaired to the Town of Hamden specification/details."

I hope that this letter helps to clarify points and concerns that were brought up during our 12/01/20 meeting. If you have any additional questions or concerns, please let me know and I will do my best to address them as quickly as possible.

Sincerely,

James DiMeo

James DiMeo, PE
Juliano Associates, LLC
405 Main Street, Wallingford CT 06492

CC: Mark Austin, PE, Town Engineer
Holly Masi, CZEO
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Judy Schuler, PE, East-West Engineering, PLLC
Robert Carr, PE, LEP, East-West Engineering, PLLC

Hydrograph Report

Hyd. No. 1

Swale

Hydrograph type	= Rational	Peak discharge	= 1.954 cfs
Storm frequency	= 10 yrs	Time to peak	= 5 min
Time interval	= 1 min	Hyd. volume	= 879 cuft
Drainage area	= 0.300 ac	Runoff coeff.	= 0.9*
Intensity	= 7.238 in/hr	Tc by User	= 5.00 min
IDF Curve	= Connecticut IDF.IDF	Asc/Rec limb fact	= 1/2

* Composite (Area/C) = [(0.300 x 0.90)] / 0.300

